



Becker County Planning & Zoning
915 Lake Ave
Detroit Lakes, MN 56501
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200492000-2025

Certificate of Compliance

Inspection Report - Permit #: SS2025-2461

Owner & Property Information

Owner Name:	DEAN SPAETH	Site Address:	39021 DORA LEE RD
Mailing Address:	DEAN SPAETH 1526 210TH ST MAHNOMEN MN 56557	Township - Sec/Twp/Rng:	MAPLE GROVE - 09/142/040
Parcel #:	200492000	Legal Description:	DORA LEE ESTATES LOT 7
Secondary Parcel #:		Designer:	JenCo Services, LLC, L4041 (James Piper)
		Installer:	Muff's Trenching LLC, L576 (Ronald Muff)

Inspector Verified Specifications

Insp- Effluent Screen Installed:	No	Insp- Tank Nbr/Size:	2/1500/2-1000
Insp- Alarm Required:	Yes	Insp- Drainfield Type:	Mound
Insp- Lift Pump in System:	Yes	Insp- Drainfield Size:	10X65 ROCK BED
Insp- Number of Bedrooms:	5	Insp- Soil Verification:	#1:SEE ATTACHED #2:N/A #3:N/A

Inspector Verified Setbacks

Insp- Tank Dist to Road	10+	Insp- Drainfield Dist to Road	10+
Insp- Tank Dist to Nearest Prop Line	10	Insp- Drainfield Dist to Nearest Prop Line	PLA/less than 10ft
Insp- Tank Dist to Nearest Structure	10	Insp- Drainfield Dist to Nearest Structure	20
Insp- Tank Dist to Well	50+	Insp- Drainfield Dist to Well	50+
Insp- Tank Dist to OHW	75+	Insp- Drainfield Dist to OHW	75+
Insp- Tank Dist to Pond/Wetland	NA	Insp- Drainfield Dist to Pond/Wetland	NA
Insp- Tank Dist to Pressure Line		Insp- Drainfield Dist to Pressure Line	

Certificate of Compliance

(Yes) Certificate is hereby granted based upon the application, addendum from, plans, specifications and all other supporting data. With proper maintenance, this system can be expected to function satisfactory, however this is not a guarantee.

Certification Date: 08/21/2025

Zoning Office Signature:

Jeff Rusness - ISTS Inspector

* Certificate of Compliance is not valid unless signed by a Registered Qualified Employee *

Field Review Form

Permit # SS2025-2461

Property and Owner

Owner: DEAN SPAETH

Parcel Number: 200492000

Site Address: 39021 DORA LEE RD

Secondary Parcel:

Home Information

Does the structure contain any of the following elements?

Designer submitted

Inspector verified

Garbage disposal: No

Garbage disposal? Y ☒ N

Dishwasher:

Dishwasher? Y ☒ N

Grinder pump:

Grinder pump? Y ☒ N

Lift pump in bsmt:

Lift pump in basement? Y ☒ N

Number of bedrooms: 5

Review - Number of bedrooms: 5

Effluent screen

Effluent screen installed? Y ☒ N Mfr:

Alarm: Yes Type: ELECTRIC

Review - Alarm? Y ☒ N Type & Mfr: Alden Power Post

Lift pump in system: Yes

Review - Lift pump in system? Y ☒ N Mfr: Zoeller N98

Component Information

Tank size: 1500, 1000

Review - Tank nbr: 2 size: 1000/1500 Mfr: Theben

Drainfield type: Mound

Review - Drainfield type: Mound

Drainfield size: Full size - 625
Reduced/warr. size -

Review - Drainfield status: none / installed next spring
Review - Drainfield size: 10x65 Rockwell

Absorption area size: 6"

Review - Absorption area size:

Chamber type/num:
Trench sqft/chamber -

Review - Chamber type: NA Num:
Review - Trench sqft/chamber:

Drainfield rock depth: 6"

Review - Rock depth: 6 in

Soil Verification

Vertical separation verified

See attached

Boring #1:

Boring #2:

Boring #3:

Setback Verification

Distance to...	Designer submitted		Inspector verified	
	Tank	Drainfield	Tank	Drainfield
Road	+50'	+10'	10	10
Nearest prop line	+20'	5'	10	PLA
Nearest structure	+10'	+20'	10	20
Well	+50'	+50'	50+	50+
OHW	+75'	+100'	75	75
Pond/Wetland			NA	NA
Pressure line			NA	NA

Date System Installed: 8-20-2025

Installer: muffs trenching

Inspector: Jeff Harsness



Preliminary Evaluation Worksheet

v 03.15.2023

1. Contact Information

Property Owner/Client: **DEAN P SPAETH & JOANN C SPAETH** Date Completed: **9/1/2024**

Site Address: **39021 DORA LEE RD, WAUBUN, MN** Project ID:

Email: **UNKNOWN** Phone: **UNKNOWN**

Mailing Address: **1526 210TH ST, MAHNOMEN, MN 56557** Alt Phone:

Legal Description: **DORA LEE ESTATES LOT 7**

Parcel ID: **200492000** SEC: **09** TWP: **142** RNG: **040**

2. Flow and General System Information

A. Client-Provided Information

Project Type: ☐ New Construction ☒ Replacement ☐ Expansion ☐ Repair

Project Use: ☒ Residential ☐ Other Establishment:

Residential use: # Bedrooms: **5** Dwelling sq.ft.: Unfinished sq.ft.:

Adults: **2** # Children: # Teenagers:

In-home business (Y/N): **No** If yes, describe:

Water-using devices: ☐ Garbage Disposal/Grinder ☒ Dishwasher ☐ Hot Tub*
☐ Sewage pump in basement ☒ Water Softener* ☐ Sump Pump*
☐ Large Bathtub >40 gallons ☐ Iron Filter* ☐ Self-Cleaning Humidifier*
☒ Clothes Washing Machine ☒ High Eff. Furnace* ☐ Other:

* Clear water source - should not go into system

Additional current or future uses: **NONE**

Anticipated non-domestic waste: **NONE**

The above is complete & accurate:

Client signature & date

B. Designer-determined Flow and Anticipated Waste Strength Information

Attach additional information as necessary.

Design Flow: **750** GPD Anticipated Waste Type: **Residential**

Maximum Concentration BOD: **170** mg/L TSS: **60** mg/L Oil & Grease: **25** mg/L

3. Preliminary Site Information

A. Water Supply Wells

#	Description	Mn. ID#	Well Depth (ft.)	Casing Depth (ft.)	Confining Layer	STA Setback	Source
1	DEEP		>50				OWNER
2							
3							
4							

Additional Well Information:



Preliminary Evaluation Worksheet

Site within 200' of noncommunity transient well (Y/N)	No	Yes, source:	
Site within a drinking water supply management area (Y/N)	No	Yes, source:	
Site in Well Head Protection inner wellhead management zone (Y/N)	No	Yes, source:	
Buried water supply pipes within 50 ft of proposed system (Y/N)	No		
B. Site located in a shoreland district/area?	Yes	Yes, name:	WHITE EARTH
Elevation of ordinary high water level:	1452 ft	Source:	GIS
Classification: Lake- Recreational	Tank Setback: 75 ft.	STA Setback:	75 ft.
C. Site located in a floodplain?	No	Yes, Type(s):	N/A
Floodplain designation/elevation (10 Year):	N/A ft	Source:	N/A
Floodplain designation/elevation (100 Year):	N/A ft	Source:	N/A
D. Property Line Id / Source:	☐ Owner	☐ Survey	☒ County GIS
			☐ Plat Map
			☐ Other:
E. ID distance of relevant setbacks on map:	☐ Water	☐ Easements	☒ Well(s)
	☒ Building(s)	☒ Property Lines	☒ OHWL
			☐ Other:

4. Preliminary Soil Profile Information From Web Soil Survey (attach map & description)

Map Units:	1138	Slope Range:	0 %
List landforms:	BEACHES ON LAKESHORES		
Landform position(s):	Back/ Side Slope		
Parent materials:	Till		
Depth to Bedrock/Restrictive Feature:	>80 in	Depth to Watertable:	>80 in
Map Unit Ratings	Septic Tank Absorption Field- At-grade:	Moderately Limited	
	Septic Tank Absorption Field- Mound:	Not Limited	
	Septic Tank Absorption Field- Trench:	Extremely Limited	

5. Local Government Unit Information

Name of LGU:	BECKER COUNTY
LGU Contact:	KYLE VAREBERG
LGU-specific setbacks:	
LGU-specific design requirements:	
LGU-specific installation requirements:	

Notes:



Field Evaluation Worksheet

1. Project Information

v 03.15.2023

Property Owner/Client: **DEAN P SPAETH & JOANN C SPAETH**

Project ID:

Site Address: **39021 DORA LEE RD, WAUBUN, MN**

Date Completed: **9/1/2024**

2. Utility and Structure Information

Utility Locations Identified ☐ Gopher State One Call #

☐ Any Private Utilities:

Locate and Verify (see *Site Evaluation map*)

☐ Existing Buildings

☐ Improvements

☐ Easements

☐ Setbacks

3. Site Information

Vegetation type(s): **Grass**

Landscape position: **Back/ Side Slope**

Percent slope: **3** %

Slope shape: **Linear, Linear**

Slope direction: **northeast**

Describe the flooding or run-on potential of site: **NONE**

Describe the need for Type III or Type IV system: **NONE**

Note:

Proposed soil treatment area protected? (Y/N): **Yes**

If yes, describe:

4. General Soils Information

Filled, Compacted, Disturbed areas (Y/N): **No**

If yes, describe:

Soil observations were conducted in the proposed system location (Y/N): **Yes**

A soil observation in the most limiting area of the proposed system (Y/N): **Yes**

Number of soil observations: **3**

Soil observation logs attached (Y/N): **Yes**

Percolation tests performed & attached (Y/N): **No**

5. Phase I. Reporting Information

	Depth	Elevation	
Limiting Condition*:	18 in	96.4 ft	*Most Restrictive Depth Identified from List Below
Periodically saturated soil:	18 in	96.4 ft	
Standing water:	>80 in	 ft	
Bedrock:	>80 in	 ft	
Benchmark Elevation:	100.0 ft		
		Elevations and Benchmark on map? (Y/N):	Yes

Soil Texture: **Medium Sand**

Percolation Rate: min/inch

Soil Hyd Loading Rate: **1.2** gpd/sq.ft

Benchmark Elevation Location: **TOP OF MOUND**

Differences between soil survey and field evaluation:

Site evaluation issues / comments:

Anticipated construction issues:

✓ 03.15.2023

(Date)

03 15 2023

39021 DORA LEE RD, WAUBUN, MN

Confidential. No

mark. 070

Conclusion:

Answer

[illegible]

Comments:

I hereby certify that I have completed this work in accordance with all applicable ordinances, rules and laws.

04/12/24

(Date)

Optional Verification: I hereby certify that this soil observation was verified according to Minn. R. 7082.0500 subp. 3 A. The signature below represents an infield verification of the periodically saturated soil or bedrock at the proposed soil treatment and dispersal site.

(Date)

Y 03.15.2023

Client: DEAN P SPAETH & JOANN C SPAETHLocation / Address: 39021 DORA LEE RD, WAUBUN, MN

Soil parent material(s): (Check all that apply)

☐ Outwash☐ Lacustrine☐ Loess☒ Till☐ Alluvium☐ Bedrock☐ Organic Matter☐ Disturbed/Fill

Landscape Position:Back/Side SlopeSlope %: 3.0Slope shape:Linear, LinearFlooding/Run-On potential: No

Vegetation:GrassSoil survey map units: 1138Surface Elevation-Relative to benchmark: 96.9

Date/Time of Day/Weather Conditions:9/1/2024PM SUNNYLimiting Layer Elevation: 95.4

Observation #/Location:#3SE CORNERObservation Type:Auger

Depth (in)	Texture	Rock Frag. %	Matrix Color(s)	Mottle Color(s)	Redox Kind(s)	Indicator(s)	Structure		
							Shape	Grade	Consistence
0-3	Medium Loamy Sand	2	10YR3/2	None	None	None	Granular	Weak	Loose
			10YR4/4	None	None	None	Granular	Weak	Loose
3-9	Medium Loamy Sand	2							
			10YR5/3	None	None	None	Granular	Weak	Loose
9-18	Medium Loamy Sand	2							
			10YR5/4	None	None	None	Blocky	Weak	Friable
18+	Sandy Clay Loam	2							

Comments:

I hereby certify that I have completed this work in accordance with all applicable ordinances, rules and laws.

JAMES PIPER

(Designer/Inspector)

(Signature)

L4041

(License #)

9/1/2024

(Date)

Optional Verification: I hereby certify that this soil observation was verified according to Minn. R. 7082.0500 subp. 3 A. The signature below represents an infield verification of the periodically saturated soil or bedrock at the proposed soil treatment and dispersal site.

(LGU/Designer/Inspector)

(Signature)

(Cert #)

(Date)



Design Summary Page

1. PROJECT INFORMATION

v 03.15.2023

Property Owner/Client: DEAN P SPAETH & JOANN C SPAETH

Project ID:

Site Address: 39021 DORA LEE RD, WAUBUN, MN

Date: 08/14/25

Email Address: UNKNOWN

Phone: UNKNOWN

2. DESIGN FLOW & WASTE STRENGTH

Attach waste strength data/estimated strength for Other Establishments

Design Flow: 750 GPD

Anticipated Waste Type: Residential

BOD: 170 mg/L TSS: 60 mg/L Oil & Grease: 25 mg/L

Treatment Level: C Select Treatment Level C for residential septic tank effluent

3. HOLDING TANK SIZING

Minimum Capacity: Residential = 1000 gal or 400 gal/bedroom, Other Establishment = Design Flow x 5.0, Minimum size 1000 gallons

Code Minimum Holding Tank Capacity: Gallons with Tanks or Compartments

Recommended Holding Tank Capacity: Gallons with Tanks or Compartments

Type of High Level Alarm: (Set @ 75% tank capacity)

Comments:

4. SEPTIC TANK SIZING

A. Residential dwellings:

Number of Bedrooms (Residential): 5

Code Minimum Septic Tank Capacity: 1500 Gallons with 1 Tanks or Compartments

Recommended Septic Tank Capacity: 1500 Gallons with 1 Tanks or Compartments

Effluent Screen & Alarm (Y/N): No Model/Type:

B. Other Establishments:

Waste received by: GPD x Days Hyd. Retention Time

Code Minimum Septic Tank Capacity: Gallons with Tanks or Compartments

Recommended Septic Tank Capacity: Gallons with Tanks or Compartments

Effluent Screen & Alarm (Y/N): Model/Type:

* Other Establishments Require Department of Labor and Industry Approval and Inspection for Building Sewer *

5. PUMP TANK SIZING

Soil Treatment Dosing Tank

Pump Tank Capacity (Minimum): 750 Gal

Pump Tank Capacity (Recommended): 1000 Gal

Pump Req: 56.0 GPM Total Head 14.8 ft

Supply Pipe Dia. 2.00 in Dose Vol: 150.0 gal

Other Component Dosing Tank:

Pump Tank Capacity (Minimum): Gal

Pump Tank Capacity (Recommended): Gal

Pump Req: GPM Total Head ft

Supply Pipe Dia. in Dose Vol: Gal

* Flow measurement device must be incorporated for any system with a pump: Elapsed Time Meter and/or Event Counter *

6. SYSTEM AND DISTRIBUTION TYPE

Project ID:

Soil Treatment Type:

Distribution Type:

Elevation Benchmark: ft

Benchmark Location:

MPCA System Type:

Distribution Media:

Type III/IV/V Details:

7. SITE EVALUATION SUMMARY:

Describe Limiting Condition:

Layers with >35% Rock Fragments? (yes/no) If yes, describe below: % rock and layer thickness, amount of soil credit and any additional information for addressing the rock fragments in this design.

Note:

	Depth	Depth	Elevation of Limiting Condition	
Limiting Condition:	18 inches	1.5 ft	96.40 ft	Critical for system compliance
Minimum Req'd Separation:	36 inches	3.0 ft		Distribution Elevation > Code Max Depth
Code Max System Depth*:	Mound inches	-1.5 ft	99.40 ft	Elevation OK

*This is the maximum depth to the bottom of the distribution media for required separation. Negative Depth (ft) requires a mound.

Designed Distribution Elevation: ft Minimum Sand Depth: inches

A. Soil Texture:

B. Organic Loading Rate (optional): lbs/sq.ft/day 0

C. Soil Hyd. Loading Rate: GPD/ft²

D. Percolation Rate: MPI

E. Contour Loading Rate:

Note:

F. Measured Land Slope: %

Note:

Comments:

8. SOIL TREATMENT AREA DESIGN SUMMARY

Trench:

Dispersal Area sq.ft Sidewall Depth in Trench Width ft

Total Lineal Feet ft No. of Trenches Code Max. Trench Depth in

Contour Loading Rate ft Minimum Length ft Designed Trench Depth in

Bed:

Dispersal Area sq.ft Sidewall Depth in Maximum Bed Depth in

Bed Width ft Bed Length ft Designed Bed Depth in

Mound:

Dispersal Area sq.ft Bed Length ft Bed Width ft

Absorption Width ft Clean Sand Lift ft Berm Width (0-1%) ft

Upslope Berm Width ft Downslope Berm ft Endslope Berm Width ft

Total System Length ft System Width ft Contour Loading Rate gal/ft



Design Summary Page

Project ID: _____

At-Grade:

Dispersal Area sq.ft Bed Length ft Bed Width ft
Upslope Berm ft Downslope Berm ft Finished Height ft
System Length ft Endslope Berm ft System Width ft

Level & Equal Pressure Distribution Soil Treatment Area

No. of Laterals 3 Lateral Diameter 2.00 in Lateral Spacing 3 ft
Perforation Spacing 2.5 ft Perforation Diameter 1/4 in Drainback Volume 5 gal
Min Dose Volume 123 gal Max Dose Volume 188 gal Total Dosing Volume 155 gal

Non-Level and Unequal Pressure Distribution Soil Treatment Area

	Elevation (ft)	Pipe Size (in)	Pipe Volume (gal/ft)	Pipe Length (ft)	Perf Size (in)	Spacing (ft)	Spacing (in)	Minimum Dose Volume gal
Lateral 1								
Lateral 2								Maximum Dose Volume gal
Lateral 3								
Lateral 4								
Lateral 5								Total Dosing Volume gal
Lateral 6								

9. Organic Loading and Additional Info for At-Risk, HSW or Type IV Design

Organic Loading to Soil Treatment

A. Starting BOD Concentration = Design Flow X 0.7 X Starting BOD (mg/L) X 8.35 ÷ 1,000,000

gpd X mg/L X 8.35 ÷ 1,000,000 = lbs. BOD/day (Organic Loading Design)

B. Organic Loading to Soil Treatment Area: (enter loading value in 7B)

mg/L X gpd X 0.7 X 8.35 ÷ 1,000,000 ÷ sq.ft = lbs./day/sqft

HSW Technology Strength Reduction

A. Starting BOD Concentration = Design Flow X Starting BOD (mg/L) X 8.35 ÷ 1,000,000

gpd X mg/L X 8.35 ÷ 1,000,000 = lbs. BOD/day (HSW Technology Design)

B. Target BOD Concentration = Design Flow X Target BOD (mg/L) X 8.35 ÷ 1,000,000

gpd X mg/L X 8.35 ÷ 1,000,000 = lbs. BOD/day (HSW Technology Design)

Lbs. BOD To Be Removed: lbs. BOD/day (HSW Technology Design)

Pretreatment Technology: *Must Meet or Exceed Target

Disinfection Technology: *Required for Levels A & B

10. Comments/Special Design Considerations:

I hereby certify that I have completed this work in accordance with all applicable ordinances, rules and laws.

JAMES PIPER

(Designer)

(Signature)

L4041

(License #)

8/14/2025

(Date)



Mound Design Worksheet

≥1% Slope

1. SYSTEM SIZING: Project ID: v 03.15.2023

- A. Design Flow: GPD
- B. Soil Loading Rate: GPD/sqft
- C. Depth to Limiting Condition: ft
- D. Percent Land Slope: %
- E. Media (Sand) Loading Rate: GPD/sqft
- F. Mound Absorption Ratio:

Table 1 MOUND CONTOUR LOADING RATES:				
Measured Perc Rate	OR	Texture - derived mound absorption ratio		Contour Loading Rate:
≤ 60mpi		1.0, 1.3, 2.0, 2.4, 2.6	→	≤ 12
61-120 mpi	OR	5.0	→	≤ 12
≥ 120 mpi*		>5.0*	→	≤ 6*

TABLE IXa LOADING RATES FOR DETERMINING BOTTOM ABSORPTION AREA AND ABSORPTION RATIOS USING PERCOLATION TESTS				
Percolation Rate (MPI)	Treatment Level C		Treatment Level A, A-2, B,	
	Absorption Area Loading Rate (gpd/ft ²)	Mound Absorption Ratio	Absorption Area Loading Rate (gpd/ft ²)	Mound Absorption Ratio
<0.1	-	1	-	1
0.1 to 5	1.2	1	1.6	1
0.1 to 5 (fine sand and loamy fine sand)	0.6	2	1	1.6
6 to 15	0.78	1.5	1	1.6
16 to 30	0.6	2	0.78	2
31 to 45	0.5	2.4	0.78	2
46 to 60	0.45	2.6	0.6	2.6
61 to 120	-	5	0.3	5.3
>120	-	-	-	-

*Systems with these values are not Type I systems.
Contour Loading Rate (linear loading rate) is a recommended value.

2. DISPERSAL MEDIA SIZING

- A. Hydraulic Absorption Required Bottom Area: Design Flow (1A) ÷ Design Media Loading Rate(1E)

$$\frac{750 \text{ GPD}}{1.2 \text{ GPD/sqft}} = 625 \text{ sq.ft}$$

Organic Sizing (OPTIONAL)

- B. Organic Absorption Bed Area = Organic Loading (Summary 9A) ÷ Organic Soil Loading Rate (Summary 7B)

$$\text{[] lbs BOD} \div \text{[] lbs BOD/sq.ft} = \text{[] sq.ft}$$

- C. Required Bed Area = Greater of Hydraulic (1D) or Organic Bed Area (1E) sq.ft

- D. Designed Dispersal Media Area: sq.ft *Optional upsizing of area to be larger than 2C*

- B. Enter Dispersal Bed Width: ft *Can not exceed 10 feet*

- C. Calculate Contour Loading Rate: Bed Width(2B) X Design Media Loading Rate(1E)

$$10 \text{ ft} \times 1.2 \text{ GPD/sqft} = 12.0 \text{ gal/ft} \quad \text{Can not exceed Table 1}$$

- D. Calculate Minimum Dispersal Bed Length: Dispersal Bed Area(2A) ÷ Bed Width(2B)

$$\frac{625 \text{ sqft}}{10.0 \text{ ft}} = 62.5 \text{ ft}$$

If a larger dispersal media Length is desired, enter size: ft

3. ABSORPTION AREA SIZING

- A. Calculate Absorption Width: Bed Width(2B) X Mound Absorption Ratio(1F)

$$10.0 \text{ ft} \times 1.0 = 10.0 \text{ ft}$$

- B. For slopes >1%, the Absorption Width is measured downhill from the upslope edge of the Bed.

Calculate Downslope Absorption Width: Absorption Width(1F) - Bed Width(2B)

$$10.0 \text{ ft} - 10.0 \text{ ft} = \text{[] ft}$$

4. DISTRIBUTION MEDIA:

Project ID:

Select Dispersal Media:

Rock

Enter Either 4A or 4B

A. Rock Depth Below Distribution Pipe

6 in

B. Registered Media

Registered Media Depth

in

Specific Media Comments:

Check registered product
information for specific
application details and design

5. MOUND SIZING

Project ID:

A. Clean Sand Lift: Required Separation - Depth to Limiting Condition = Clean Sand Lift (1 ft minimum)
 $3.0 \text{ ft} - 1.5 \text{ ft} = 1.5 \text{ ft}$ Design Sand Lift (optional): ft
B. Upslope Height: Clean Sand Lift(6A) + Depth of Media(4AorB) + Depth to Cover Pipe + Depth of Cover (1 ft)
 $1.5 \text{ ft} + 0.50 \text{ ft} + 0.33 \text{ ft} + 1.0 \text{ ft} = 3.3 \text{ ft}$

Land Slope %	0	1	2	3	4	5	6	7	8	9	10	11	12
Upslope Berm Ratio 3:1	3.00	2.91	2.83	2.75	2.68	2.61	2.54	2.48	2.42	2.36	2.31	2.26	2.21
Upslope Berm Ratio 4:1	4.00	3.85	3.70	3.57	3.45	3.33	3.23	3.12	3.03	2.94	2.86	2.78	2.70

C. Select Upslope Berm Multiplier (based on land slope):

3.57

D. Calculate Upslope Berm Width: Multiplier (5C) X Upslope Mound Height (5B)
 $3.57 \times 3.3 \text{ ft} = 11.9 \text{ ft}$
E. Calculate Drop in Elevation Under Bed: Bed Width(2B) X Land Slope(1D) ÷ 100 = Drop (ft)
 $10.0 \text{ ft} \times 3.0 \% \div 100 = 0.30 \text{ ft}$
F. Calculate Downslope Mound Height: Upslope Height(5B) + Drop in Elevation(5E)
 $3.3 \text{ ft} + 0.30 \text{ ft} = 3.6 \text{ ft}$

Land Slope %	0	1	2	3	4	5	6	7	8	9	10	11	12
Downslope Berm Ratio 3:1	3.00	3.09	3.19	3.30	3.41	3.53	3.66	3.80	3.95	4.11	4.29	4.48	4.69
Downslope Berm Ratio 4:1	4.00	4.17	4.35	4.54	4.76	5.00	5.26	5.56	5.88	6.25	6.67	7.14	7.69

G. Select Downslope Berm Multiplier (based on land slope):

4.54

H. Calculate Downslope Berm Width: Downslope Multiplier(5G) X Downslope Height (5F)
 $4.54 \times 3.6 \text{ ft} = 16.5 \text{ ft}$
I. Calculate Minimum Berm to Cover Absorption Area: Downslope Absorption Width(3A) + 4 feet
 $\text{ft} + 4 \text{ ft} = 4.0 \text{ ft}$
J. Design Downslope Berm = greater of 5H and 5I:

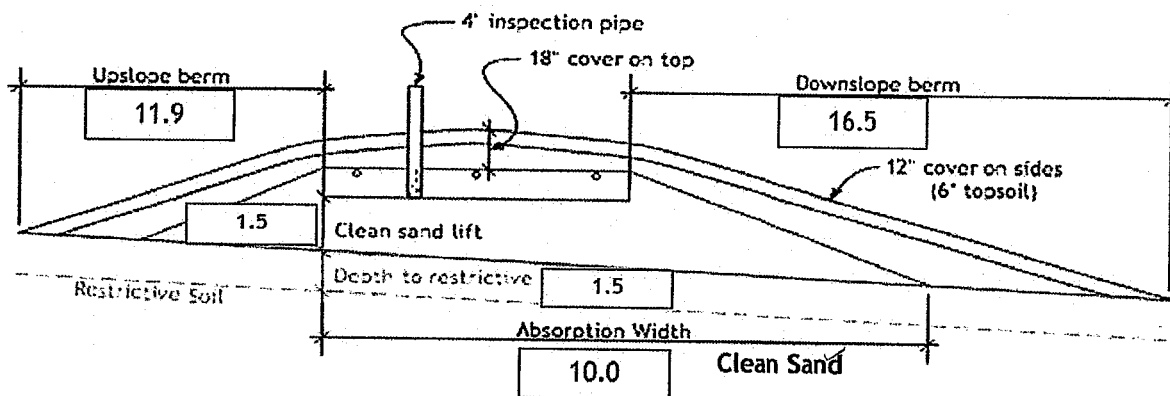
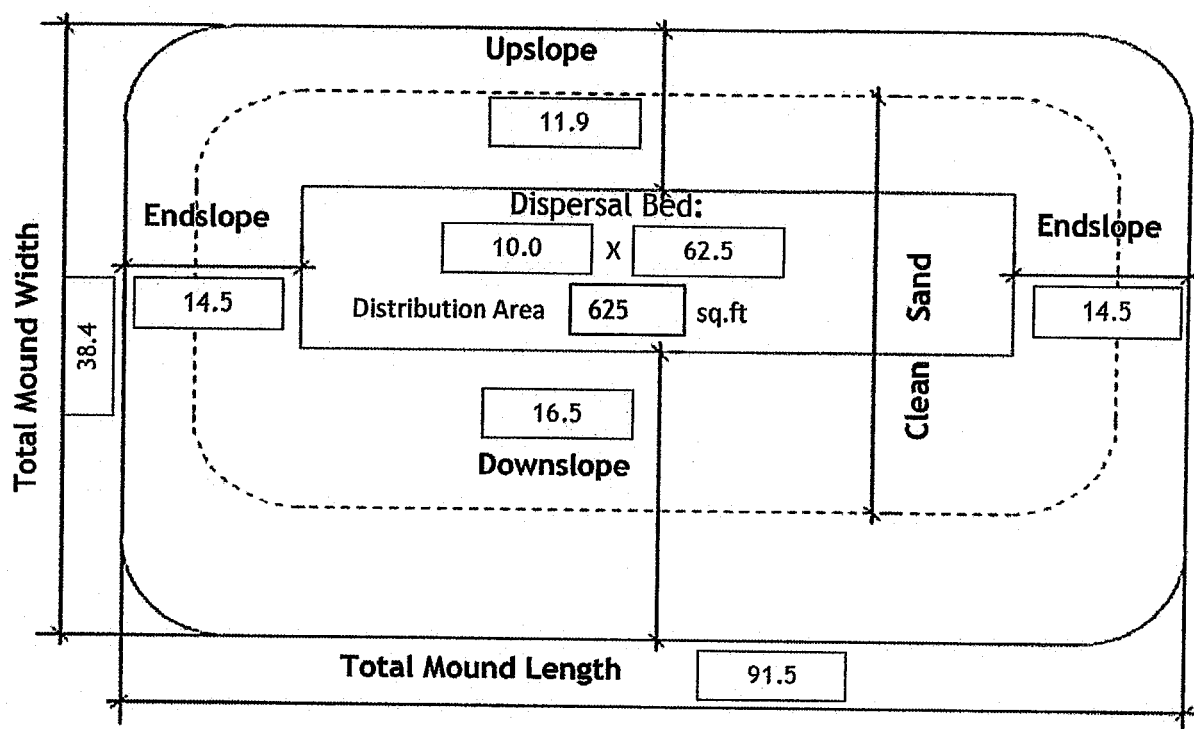
16.5 ft

K. Select Endslope Berm Multiplier:

4.00

(usually 3.0 or 4.0)

L. Calculate Endslope Berm Width = Endslope Berm Multiplier(5K) X Downslope Mound Height(5F)
 $4.00 \times 3.6 \text{ ft} = 14.5 \text{ ft}$
M. Calculate Mound Width: Upslope Berm Width(5D) + Bed Width(2B) + Downslope Berm Width(5J)
 $11.9 \text{ ft} + 10.0 \text{ ft} + 16.5 \text{ ft} = 38.4 \text{ ft}$
N. Calculate Mound Length: Endslope Berm Width (5L) + Bed Length(2D) + Endslope Berm Width(5L)
 $14.5 \text{ ft} + 62.5 \text{ ft} + 14.5 \text{ ft} = 91.5 \text{ ft}$



Required Separation: 36 (in)

Distribution Media: Rock

Media Depth: 6.0 (in)

Manifold Connection: End

Lateral Pipe Diameter: 2.00 (in)

Perforation Size: 1/4 (in)

Elevation to Benchmark

Elevation Limiting Layer: 96.4 ft

Elevation required Separation: 99.4 ft

Elevation Distribution Media Bottom: 99.4 ft

Elevation Top of Media(min): 100.2 ft

Elevation Top of System(min): 101.2 ft

Perforation Spacing: 30.0 (in)

If Split and Non-Level Pressure Distribution Used: See Non-Level Pressure Distribution Form

Comments:



Mound Materials Worksheet

Project ID:

v 03.15.2023

A. Rock Volume : (Rock Below Pipe + Rock to cover pipe (pipe outside dia + -2 inch)) X Bed Length X Bed Width = Volume

$$(\boxed{6} \text{ in} + \boxed{4.0} \text{ in}) \div 12 \times \boxed{62.5} \text{ ft} \times \boxed{10.0} \text{ ft} = \boxed{520.8} \text{ cu.ft}$$

$$\text{Divide cu.ft by 27 cu.ft/cu.yd to calculate cubic yards: } \boxed{520.8} \text{ cu.ft} \div 27 = \boxed{19.3} \text{ cu.yd}$$

$$\text{Add 30% for constructability: } \boxed{19.3} \text{ cu.yd} \times 1.3 = \boxed{25.1} \text{ cu.yd}$$

B. Calculate Clean Sand Volume:

Volume Under Rock bed : Average Sand Depth x Media Width x Media Length = cubic feet

$$\boxed{1.7} \text{ ft} \times \boxed{10.0} \text{ ft} \times \boxed{62.5} \text{ ft} = \boxed{1031} \text{ cu.ft}$$

For a Mound on a slope from 0-1%

Volume from Length = ((Upslope Mound Height - 1) X Absorption Width Beyond Bed X Media Bed Length)

$$\boxed{} \text{ ft} - 1) \times \boxed{} \times \boxed{} \text{ ft} = \boxed{}$$

Volume from Width = ((Upslope Mound Height - 1) X Absorption Width Beyond Bed X Media Bed Width)

$$\boxed{} \text{ ft} - 1) \times \boxed{} \times \boxed{} \text{ ft} = \boxed{}$$

Total Clean Sand Volume : Volume from Length + Volume from Width + Volume Under Media

$$\boxed{} \text{ cu.ft} + \boxed{} \text{ cu.ft} + \boxed{} \text{ cu.ft} = \boxed{} \text{ cu.ft}$$

For a Mound on a slope greater than 1%

Upslope Volume : ((Upslope Mound Height - 1) x 3 x Bed Length) ÷ 2 = cubic feet

$$((\boxed{3.3} \text{ ft} - 1) \times 3.0 \text{ ft} \times \boxed{62.5}) \div 2 = \boxed{218.4} \text{ cu.ft}$$

Downslope Volume : ((Downslope Height - 1) x Downslope Absorption Width x Media Length) ÷ 2 = cubic feet

$$((\boxed{3.6} \text{ ft} - 1) \times \boxed{} \text{ ft} \times \boxed{62.5}) \div 2 = \boxed{} \text{ cu.ft}$$

Endslope Volume : (Downslope Mound Height - 1) x 3 x Media Width = cubic feet

$$(\boxed{3.6} \text{ ft} - 1) \times 3.0 \text{ ft} \times \boxed{10.0} \text{ ft} = \boxed{78.9} \text{ cu.ft}$$

Total Clean Sand Volume : Upslope Volume + Downslope Volume + Endslope Volume + Volume Under Media

$$\boxed{218.4} \text{ cu.ft} + \boxed{} \text{ cu.ft} + \boxed{78.9} \text{ cu.ft} + \boxed{1031.3} \text{ cu.ft} = \boxed{1328.6} \text{ cu.ft}$$

$$\text{Divide cu.ft by 27 cu.ft/cu.yd to calculate cubic yards: } \boxed{1328.6} \text{ cu.ft} \div 27 = \boxed{49.2} \text{ cu.yd}$$

$$\text{Add 30% for constructability: } \boxed{49.2} \text{ cu.yd} \times 1.3 = \boxed{64.0} \text{ cu.yd}$$

C. Calculate Sandy Berm Volume:

Total Berm Volume (approx.) : ((Avg. Mound Height - 0.5 ft topsoil) x Mound Width x Mound Length) ÷ 2

$$(\boxed{3.5} - 0.5) \text{ ft} \times \boxed{38.4} \text{ ft} \times \boxed{91.5} \div 2 = \boxed{5233.2} \text{ cu.ft}$$

Total Mound Volume - Clean Sand volume - Rock Volume = cubic feet

$$\boxed{5233.2} \text{ cu.ft} - \boxed{1328.6} \text{ cu.ft} - \boxed{520.8} \text{ cu.ft} = \boxed{3383.8} \text{ cu.ft}$$

$$\text{Divide cu.ft by 27 cu.ft/cu.yd to calculate cubic yards: } \boxed{3383.8} \text{ cu.ft} \div 27 = \boxed{125.3} \text{ cu.yd}$$

$$\text{Add 30% for constructability: } \boxed{125.3} \text{ yd}^3 \times 1.3 = \boxed{162.9} \text{ cu.yd}$$

D. Calculate Topsoil Material Volume: Total Mound Width X Total Mound Length X .5 ft

$$\boxed{38.4} \text{ ft} \times \boxed{91.5} \text{ ft} \times 0.5 \text{ ft} = \boxed{1756.1} \text{ cu.ft}$$

$$\text{Divide cu.ft by 27 cu.ft/cu.yd to calculate cubic yards: } \boxed{1756.1} \text{ cu.ft} \div 27 = \boxed{65.0} \text{ cu.yd}$$

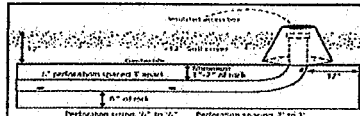
$$\text{Add 30% for constructability: } \boxed{65.0} \text{ cu.yd} \times 1.3 = \boxed{84.6} \text{ cu.yd}$$

Pressure Distribution Design Worksheet

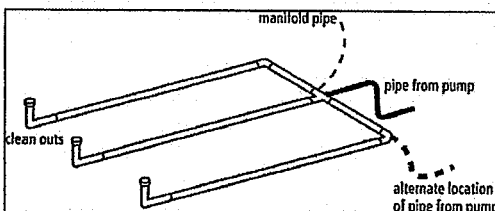
Project ID:

v 03.15.2023

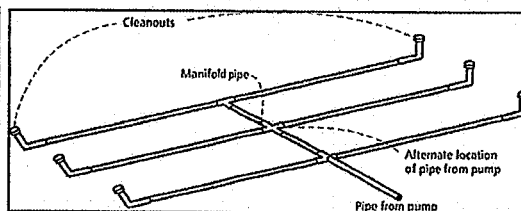
- Media Bed Width: ft
- Minimum Number of Laterals in system/zone = Rounded up number of $[(\text{Media Bed Width} - 4) \div 3] + 1$.
 $[(\text{10} - 4) \div 3] + 1 = \text{3}$ laterals *Does not apply to at-grades*
- Designer Selected Number of Laterals: laterals
Cannot be less than line 2 (Except in at-grades)
- Select Perforation Spacing: ft
- Select Perforation Diameter Size: in
- Length of Laterals = Media Bed Length(1.) - 2 Feet.
 $62.5 - 2\text{ft} = 60.5$ ft *Perforation can not be closer then 1 foot from edge.*
- Determine the Number of Perforation Spaces. Divide the Length of Laterals(6.) by the Perforation Spacing(4.) and round down to the nearest whole number.
 Number of Perforation Spaces = $60.5 \text{ ft} \div 2.5 \text{ ft} = 24$ Spaces
- Number of Perforations per Lateral is equal to 1.0 plus the Number of Perforation Spaces(7.). Check table below to verify the number of perforations per lateral guarantees less than a 10% discharge variation. The value is double with a center manifold.
 Perforations Per Lateral = $24 \text{ Spaces} + 1 = 25$ Perfs. Per Lateral



Maximum Number of Perforations Per Lateral to Guarantee <10% Discharge Variation										
1/4 Inch Perforations						7/32 Inch Perforations				
Perforation Spacing (Feet)	Pipe Diameter (Inches)					Perforation Spacing (Feet)	Pipe Diameter (Inches)			
	1	1 1/4	1 1/2	2	3		1	1 1/4	1 1/2	2
2	10	13	18	30	60	2	11	16	21	34
2 1/2	8	12	16	28	54	2 1/2	10	14	20	32
3	8	12	16	25	52	3	9	14	19	30
3/16 Inch Perforations						1/8 Inch Perforations				
Perforation Spacing (Feet)	Pipe Diameter (Inches)					Perforation Spacing (Feet)	Pipe Diameter (Inches)			
	1	1 1/4	1 1/2	2	3		1	1 1/4	1 1/2	2
2	12	18	26	46	87	2	21	33	44	74
2 1/2	12	17	24	40	80	2 1/2	20	30	41	69
3	12	16	22	37	75	3	20	29	38	64



END Connection



CENTER Connection

Perf Per Lateral:

Perf Per Lateral Equal Split: |

OPTIONAL Perf Per Lateral Non-Equal Split*: |
 * must not exceed maximum number perfs per lateral in table

9. Total Number of Perforations equals the Number of Perforations per Lateral (8.) multiplied by the Number of Perforated Laterals.(3.)

Perf. Per Lat. X Number of Perf. Lat. = Total Number of Perf.

10. Spacing of laterals; Must be greater than 1 foot and no more than 3 feet: ft

11. Select Type of Manifold Connection (End or Center):

If Center Manifold Connection the max number of perfs per lateral in the table can be doubled.

12. Select Lateral Diameter (See Table):

in

13. Calculate the Square Feet per Perforation.

Recommended value is 4-11 ft² per perforation, Does not apply to At-Grades

- a. Bed Area = Bed Width (ft) X Bed Length (ft)

10 ft X 63 ft = 625 sq.ft

- b. Square Foot per Perforation = Bed Area ÷ by the Total Number of Perfs

625 sqft ÷ 75 perf = 8.3 sq.ft/perf

14. Select Minimum Average Head:

1.0 ft

15. Select Perforation Discharge based on Table:

0.74 GPM per Perf

16. Flow Rate = Total Number of Perfs(9.) X Perforation Discharge(15.)

75 Perfs X 0.74 GPM per Perforation = 56 GPM

17. Volume of Liquid Per Foot of Distribution Piping (Table II):

0.170 Gallons/ft

18. Volume of Distribution Piping = Number of Perforated Laterals(3.) X Length of Laterals(6.) X Volume of Liquid Per Foot of Distribution Piping (17.)

3 X 61 ft X 0.170 gal/ft = 30.9 Gallons

19. Minimum Delivered Volume = Volume of Distribution Piping X 4

30.9 gals X 4 = 123.4 Gallons

20. Maximum Delivered Volume = Design flow x 25%

750.0 gpd X 25% = 187.5 Gallons

21. Minimum Delivered vs Maximum Delivered evaluation:

Volume ratio correct

Perforation Discharge (GPM)				
Head (ft)	Perforation Diameter			
	1/4	3/16	7/32	1/4
1.0'	0.18	0.41	0.56	0.74
1.5	0.22	0.51	0.69	0.9
2.0'	0.26	0.59	0.80	1.04
2.5	0.29	0.65	0.89	1.17
3.0	0.32	0.72	0.98	1.28
4.0	0.37	0.83	1.13	1.47
5.0'	0.41	0.92	1.26	1.65
1 foot	Dwellings with 3/16 inch to 1/4 inch perforations			
2 feet	Dwellings with 1/8 inch perforations			
5 feet	Other establishments and MSTs with 3/16 inch to 1/4 inch perforations			
	Other establishments and MSTs with 1/8 inch perforations			

Table II
Volume of Liquid in
Pipe

Pipe Diameter (inches)	Liquid Per Foot (Gallons)
1	0.045
1.25	0.078
1.5	0.110
2	0.170
3	0.380
4	0.661

Comments/Special Design Considerations:

1. PUMP CAPACITY		Project ID:	v 03.15.2023
Pumping to Gravity or Pressure Distribution:		Pressure	
A. If pumping to gravity enter the gallon per minute of the pump:	56.0 GPM (10 - 45 gpm)		
B. If pumping to a pressurized distribution system:	56.0 GPM		
C. Enter pump description:	Demand Dosing		

2. HEAD REQUIREMENTS		
A. Elevation Difference between pump and point of discharge:	7 ft	
B. Distribution Head Loss:	5 ft	
C. Additional Head Loss*: (due to special equipment, etc.)	ft	

* Common additional head loss: gate valve = 1 ft each, globe valve = 1.5 ft each, splitter valve = see manufacturers details

Distribution Head Loss	
Gravity Distribution = 0ft	
Pressure Distribution based on Minimum Average Head Value on Pressure Distribution Worksheet:	
Minimum Average Head	Distribution Head Loss
1ft	5ft
2ft	6ft
5ft	10ft

Table I. Friction Loss in Plastic Pipe per 100ft				
Flow Rate (GPM)	Pipe Diameter (inches)			
	1	1.25	1.5	2
10	9.1	3.1	1.3	0.3
12	12.8	4.3	1.8	0.4
14	17.0	5.7	2.4	0.6
16	21.8	7.3	3.0	0.7
18		9.1	3.8	0.9
20		11.1	4.6	1.1
25		16.8	6.9	1.7
30		23.5	9.7	2.4
35			12.9	3.2
40			16.5	4.1
45			20.5	5.0
50				6.1
55				7.3
60				8.6
65				10.0
70				11.4
75				13.0
85				16.4
95				20.1

D. 1. Supply Pipe Diameter:	2.0 in		
2. Supply Pipe Length:	30 ft		
E. Friction Loss in Plastic Pipe per 100ft from Table I:			
Friction Loss =	7.53 ft per 100ft of pipe		
F. Determine <i>Equivalent Pipe Length</i> from pump discharge to soil dispersal area discharge point. Estimate by adding 25% to supply pipe length for fitting loss. <i>Supply Pipe Length X 1.25 = Equivalent Pipe Length</i>			
30	ft	X	1.25 = 37.5 ft
G. Calculate <i>Supply Friction Loss</i> by multiplying <i>Friction Loss Per 100ft(E.)</i> by the <i>Equivalent Pipe Length(F.)</i> and divide by 100.			
Supply Friction Loss =	7.53 ft per 100ft X 37.5 ft + 100 = 2.8 ft		
H. <i>Total Head</i> requirement is the sum of the <i>Elevation Difference(2A)</i> + <i>Distribution Head Loss(2B)</i> + <i>Additional Head Loss(2C)</i> + <i>Supply Friction Loss(2G)</i>			
7.0	ft	+	5.0 ft + 2.8 ft = 14.8 ft

3. PUMP SELECTION	
A pump must be selected to deliver at least	56.0 GPM with at least 14.8 feet of total head.
Comments:	

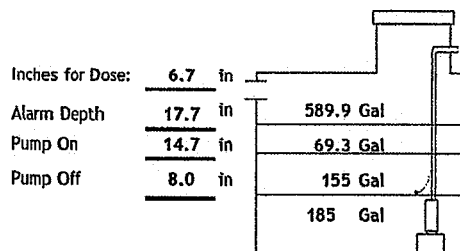


STA Dosing Pump Tank Design Worksheet (Demand Dose)

MINNESOTA POLLUTION CONTROL AGENCY

DETERMINE TANK CAPACITY AND DIMENSIONS				Project ID:	v 03.15.2023
1.	A. Design Flow (Design Sum. 1A): B. Min. required pump tank capacity:	750 GPD 750 Gal	C. Tank Use: D. Recommended pump tank capacity:	Dosing 1000 Gal	
2.	A. Tank Manufacturer: C. Capacity from manufacturer: D. Gallons per inch from manufacturer: E. Liquid depth of tank from manufacturer:	BROWN PRECAST 1001 Gallons 23.1 Gallons per inch 43.3 inches	B. Tank Model:	1000 PT	<i>Note: Design calculations are based on this specific tank. Substituting a different tank model will change the pump float or timer settings. Contact designer if changes are necessary.</i>
DETERMINE DOSING VOLUME					
3. Calculate Volume to Cover Pump (The inlet of the pump must be at least 4-inches from the bottom of the pump tank & 2 inches of water covering the pump is recommended) (Pump and block height + 2 inches) X Gallons Per Inch (2D) (6 in + 2 inches) X 23.1 Gallons Per Inch = 185 Gallons					
4. Minimum Delivered Volume = 4 X Volume of Distribution Piping: -Item 19 of the Pressure Distribution STA or Item 11 of Non-level STA 123 Gallons (Minimum dose) 5.3 inches/dose					
5. Calculate Maximum Pumpout Volume (25% of Design Flow(1A)) Design Flow: 750 GPD X 0.25 = 188 Gallons (Maximum dose) 8.1 inches/dose					
6. Select a pumpout volume that meets both Minimum and Maximum: 150 Gallons					
7. Calculate Doses Per Day = Design Flow(1A) ÷ Delivered Volume(6.) 750 gpd ÷ 150 gal = 5.00 Doses* * Doses need to be equal to or greater than 4					
8. Calculate Drainback: A. Diameter of Supply Pipe = 2 inches B. Length of Supply Pipe = 30 feet C. Volume of Liquid Per Lineal Foot of Pipe = 0.170 Gallons/ft D. Drainback = Length of Supply Pipe(8B) X Volume of Liquid Per Lineal Foot of Pipe(8C) 30 ft X 0.170 gal/ft = 5.1 Gallons					
9. Total Dosing Volume = Delivered Volume(6.) + Drainback (8D) 150 gal + 5.1 gal = 155 Gallons					
10. Minimum Alarm Volume = Depth of alarm (2 or 3 inches) X gallons per inch of tank(2D) 3 in X 23.1 gal/in = 69.3 Gallons					
11. Reserve Capacity Volume = [Tank Liquid Depth(2E) - Alarm Float Depth(10.)] x gallons per inch of tank(2D) [43.3 in - 17.7 in] X 23.1 gal/in = 589.9 Gallons					
DEMAND DOSE FLOAT SETTINGS Alarm and Pump are to be wired on separate circuits and inspected by the electrical inspector					
12. Calculate Float Separation Distance using Dosing Volume. Total Dosing Volume(9.) ÷ Gallons Per Inch(2D) 155 gal ÷ 23.1 gal/in = 6.7 inches					
13. Measuring from bottom of tank: A. Distance to set Pump Off Float = Pump + block height + 2 inches 6 in + 2 in = 8 inches B. Distance to set Pump On Float = Distance to Set Pump-Off Float(13A) + Float Separation Distance(12.) 8 in + 6.7 in = 15 inches C. Distance to set Alarm Float = Distance to set Pump-On Float(13B) + Alarm Depth (2-3 inches)(10.) 15 in + 3.0 in = 18 inches					

Volume of Liquid in Pipe	
Pipe Diameter (inches)	Liquid Per Foot (Gallons)
1	0.045
1.25	0.078
1.5	0.110
2	0.170
3	0.380
4	0.661



1. Tank Specifications

Project ID:

v 03.15.2023

A. Tank Manufacturer: **BROWN PRECAST** Tank Model: **1500 ST**

B. Outside Tank Dimensions and Specifications: Tank Use: **Septic**

Length: **144** in Width: **68** in Height: **61** in Diameter: in

Length: **12.0** ft Width: **5.7** ft Height: **5.1** ft Radius of Tank: in

2. Outside Volume of Tank

Rectangular Tank	Circular Tank
A. Area of Tank = Length (ft) X Width (ft) 12.0 ft X 5.7 ft = 68.0 sq.ft	A. Area of Tank = $\pi r^2 = (3.14 \times (\text{Radius of Tank})^2)$ 3.14 X (ft) ² = sq.ft
B. Volume of Tank = Area of Tank (2.A) X Height (ft) 68.0 sq.ft X 5.1 ft = 345.7 cu.ft	B. Volume of Tank = Area of Tank X Height (ft) sq.ft X ft = cu.ft

3. Force of Tank Weight (F_{TW})

Weight of Tank (provided by manufacturer) **13000** lbs

4. Force of Soil Weight Over Tank (F_{SW})

A. Depth of Cover Over Tank: **24** in **2.0** ft

B. Weight of Soil Per Cubic Foot: **120** lbs/cu.ft

C. Volume of Soil Over Tank = Depth of Cover(4A) (ft) X Area of Tank(2A) (ft²)
2.0 ft X **68.0** sq.ft = **136.0** cu.ft

D. Weight of Soil Over Tank = Volume of Soil Over Tank(4C) X Weight of Soil Per Cubic Foot
136.0 cu.ft X **120** lbs/cu.ft = **16,320.0** lbs

Note: Assumes saturation does not get over the lid of the tank

Soil Type	Weight of Soil (lbs/ft ³)
Sandy	120
Loamy	100
Clay	90

5. Buoyant Force (F_B)

Buoyant Force (F_B) = Outside Volume of Tank(2B) X Weight of Water Per Cubic Foot (62.4 lbs/ft³) X 1.2 (Safety Factor)

346 X 62.4 lbs/cu.ft X 1.2 = **25,883.5** lbs

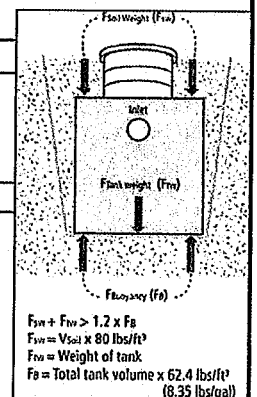
6. Evaluation of Net Forces

A. Downward Force = Force of Tank Weight (F_{TW})(3.) + Force of Soil Weight of Soil (F_{SW})(4.)
13000 lbs + **16320** lbs = **29,320.0** lbs

B. Net Difference = Downward Force(6A) - Buoyant Force Including Safety Factor (5.)
29320 lbs - **25884** lbs = **3,436.5** lbs

If the Net Difference is negative, counter measures will need to be taken to prevent the tank from floating out of the ground.

Comments/Solution:



1. Tank Specifications

Project ID:

v 03.15.2023

A. Tank Manufacturer: **BROWN PRECAST**

Tank Model: **1000 PT**

B. Outside Tank Dimensions and Specifications:

Tank Use: **Dosing**

Length: **86** in Width: **68** in Height: **61.25** in

Diameter: in

Length: **7.2** ft Width: **5.7** ft Height: **5.1** ft

Radius of Tank: in

2. Outside Volume of Tank

Rectangular Tank

A. Area of Tank = Length (ft) X Width (ft)

7.2 ft X **5.7** ft = **40.6** sq.ft

B. Volume of Tank = Area of Tank (2.A) X Height (ft)

40.6 sq.ft X **5.1** ft = **207.3** cu.ft

Circular Tank

A. Area of Tank = $\pi r^2 = (3.14 \times (\text{Radius of Tank})^2)$

3.14 X (ft)² = sq.ft

B. Volume of Tank = Area of Tank X Height (ft)

sq.ft X ft = cu.ft

3. Force of Tank Weight (F_{TW})

Weight of Tank (provided by manufacturer) **8650** lbs

4. Force of Soil Weight Over Tank (F_{SW})

A. Depth of Cover Over Tank: **24** in **2.0** ft

B. Weight of Soil Per Cubic Foot: **120** lbs/cu.ft

C. Volume of Soil Over Tank = Depth of Cover(4A) (ft) X Area of Tank(2A) (ft²)

2.0 ft X **40.6** sq.ft = **81.2** cu.ft

D. Weight of Soil Over Tank = Volume of Soil Over Tank(4C) X Weight of Soil Per Cubic Foot

81.2 cu.ft X **120** lbs/cu.ft = **9,746.7** lbs Note: Assumes saturation does not get over the lid of the tank

Soil Type	Weight of Soil (lbs/ft ³)
Sandy	120
Loamy	100
Clay	90

5. Buoyant Force (F_B)

Buoyant Force (F_B) = Outside Volume of Tank(2B) X Weight of Water Per Cubic Foot (62.4 lbs/ft³) X 1.2 (Safety Factor)

207 X 62.4 lbs/cu.ft X 1.2 = **15,521.6** lbs

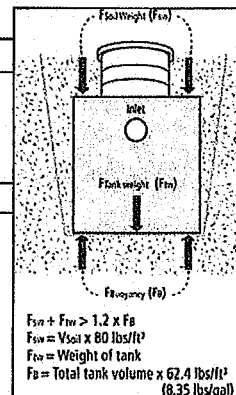
6. Evaluation of Net Forces

A. Downward Force = Force of Tank Weight (F_{TW})(3.) + Force of Soil Weight of Soil (F_{SW})(4.)

8650 lbs + **9747** lbs = **18,396.7** lbs

B. Net Difference = Downward Force(6A) - Buoyant Force Including Safety Factor (5.)

18397 lbs - **15522** lbs = **2,875.1** lbs



If the Net Difference is negative, counter measures will need to be taken to prevent the tank from floating out of the ground.

Comments/Solution:

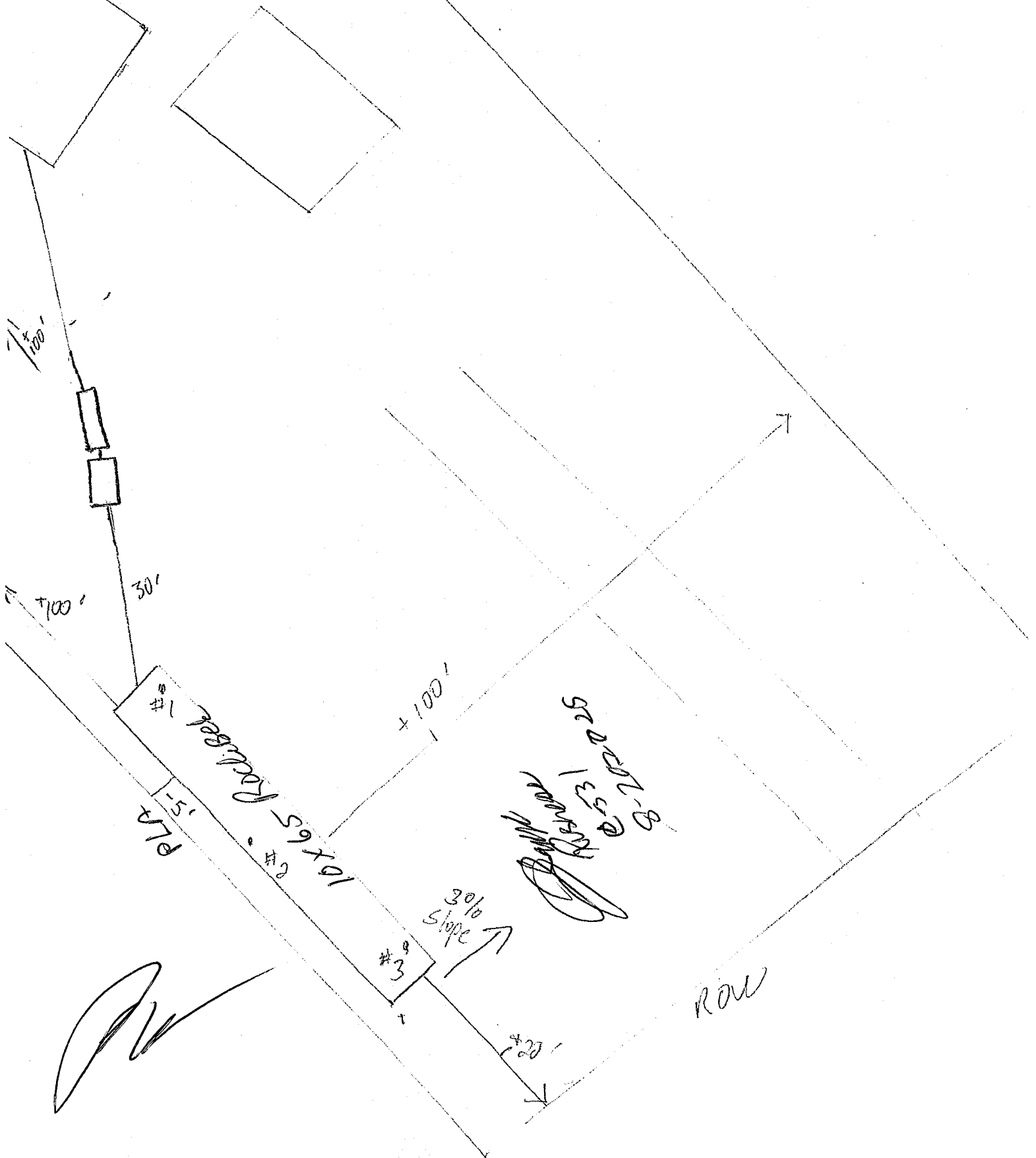
1" = 20' N

$1'' = 20'$ $\nearrow$ N

$1'' = 20'$ $\nearrow$ N

$1'' = 20'$ $\nearrow$ N

$1'' = 20'$ $\nearrow$ N



Property Line Agreement

I, Julia Ann Miller, as Trustee of the Julia Ann Miller Revocable Living Trust, owner of the property described as:

Parcel No. 20.0491.000

Property Address: 39003 Dora Lee Road, Waubun MN 56589

Lot Six (6), Block One, Dora Lee Estates, according to the certified Plat thereof on file and of record in the office of the County Recorder in and for Becker County, Minnesota.

give Dean P. Spaeth and JoAnn C. Spaeth, husband and wife, as joint tenants, owners of the property described as:

Parcel No. 20.0492.000

Property Address: 39021 Dora Lee Road, Waubun MN 56589

Lot Seven (7), Block One, Dora Lee Estates, according to the certified Plat thereof on file and of record in the office of the County Recorder in and for Becker County, Minnesota.

permission to have their sewer system closer than the required 10 feet to the lot line.

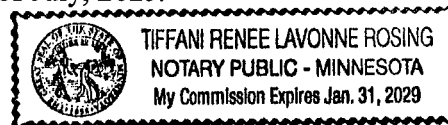
Dated: July 24, 2025

Julia Ann Miller - Trustee
Julia Ann Miller, Trustee

Subscribed and sworn to before me this 24th day of July, 2025.

Tiffany Renee Lavonne Rosing
Notary Public

My Comm. Expires: 11/31/29



(Notary Stamp)